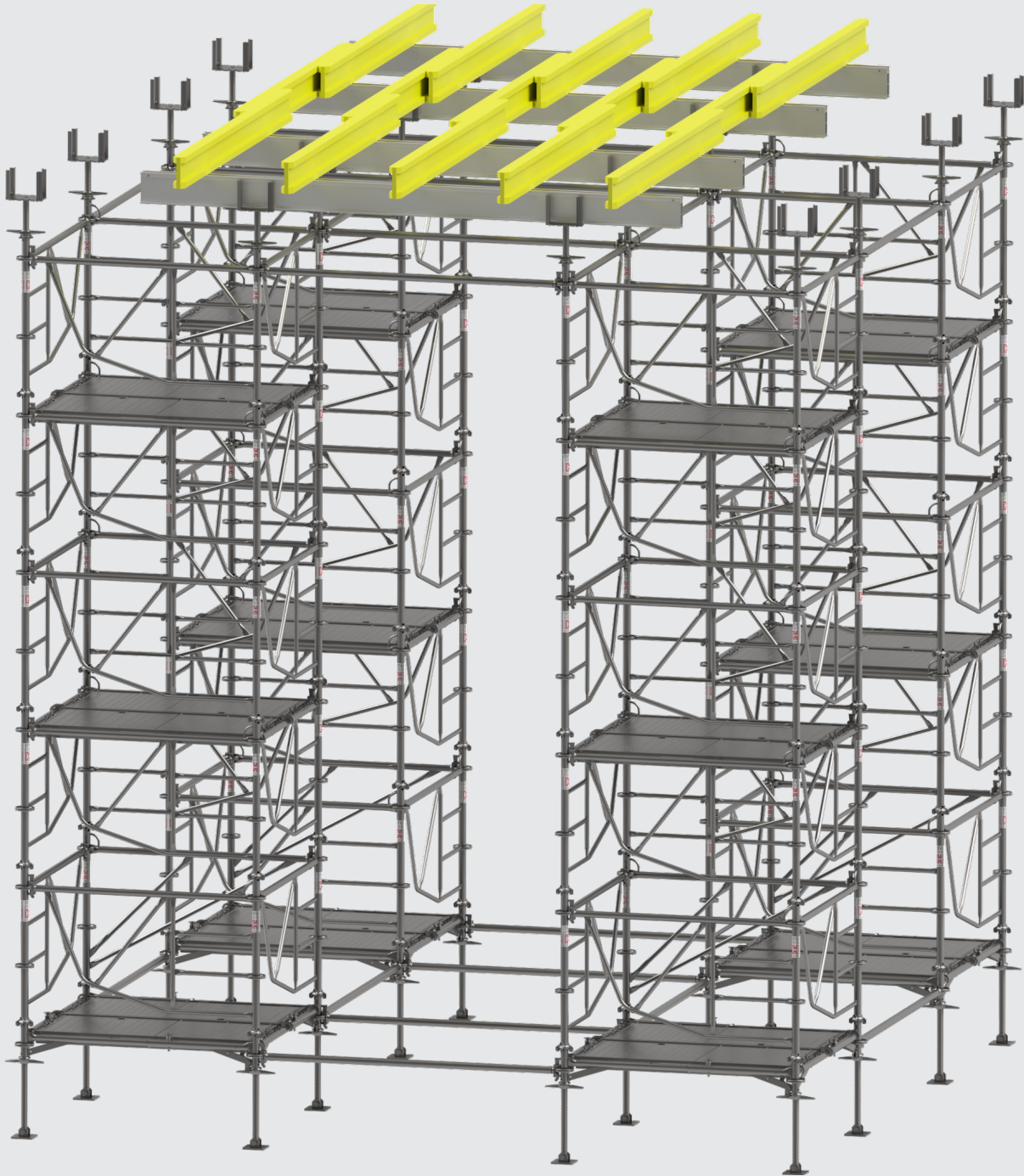


# HST 60

## Shoring Tower

### Load Tables



Keep for later use!

## Disclaimer

ST 60 load tables can be used for preliminary structural layout of ST 60 falsework. The tables presented in this document comprise 3 tower layouts: 1.13 m x 1.13 m, 1.50 m x 1.50 m and 1.13 m x 1.50 m. Secondary beams are in every case timber H20 beams, primary beams are either double timber beams H20 or single DU-AL T200 aluminium beams.

According to EN 12812 deflection of secondary and primary beams was calculated using characteristic (unfactored) loads.

For both secondary and primary beams the deflection limit value is **L/500**.

All planned vertical loads were taken into account when determining deflection, including live load and concrete heaping.

The data in the tables was calculated based on the following static schemes:

- Secondary beams as single-span beams or three-span beams
- Primary beams as single-span beams with cantilevers at both ends



## WARNING

### Risk of death or injury due to collapsing scaffold!

The resulting loads  $N_k$  [kN] on single props of the ST 60 tower shall not exceed the permitted vertical prop loads depicted in diagrams in the ST 60 user guide. Otherwise the distances between the towers have to be reduced!

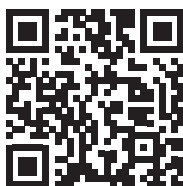
When designing falsework, the user must provide:

- Proof of stability at all stages
- Load bearing capacity
- Tilt safety (especially relevant for free standing towers in empty state)
- Sliding safety

All designs created on the basis of this document must be checked by the user prior to releasing any outputs.

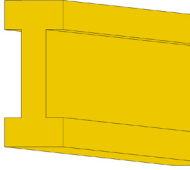
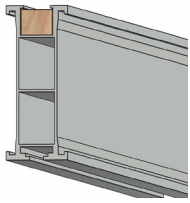
Only the user is responsible for the calculations and design safety, accuracy and completeness.

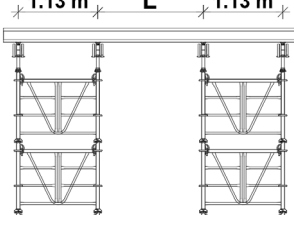
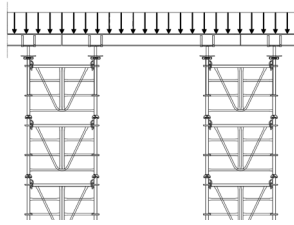
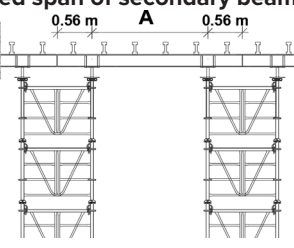
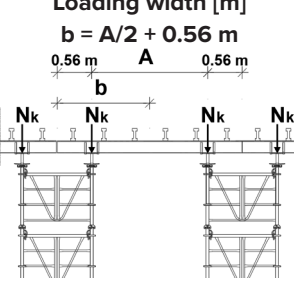
The user must always comply with the ST60 user guide. It can be downloaded from the download area at <https://huennebeck.com/>.

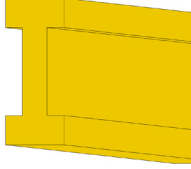


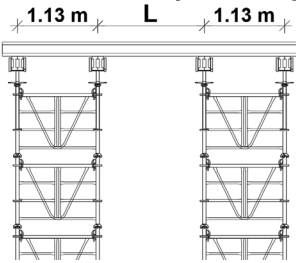
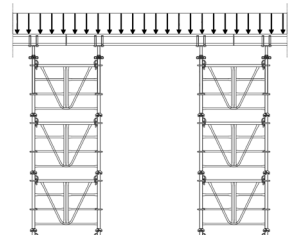
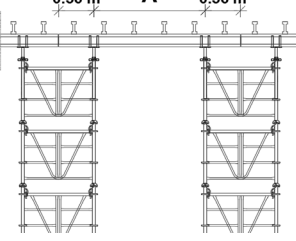
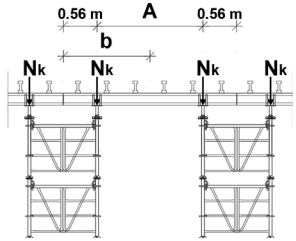
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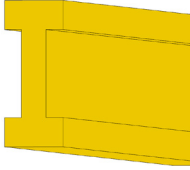
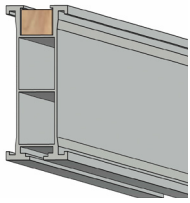
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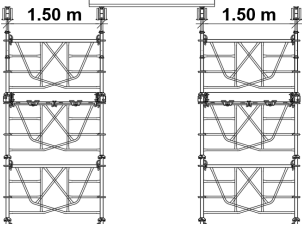
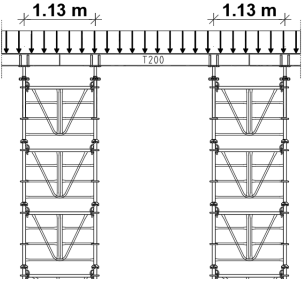
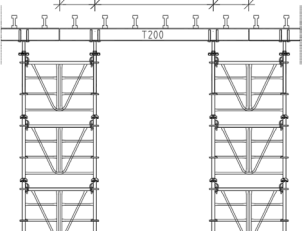
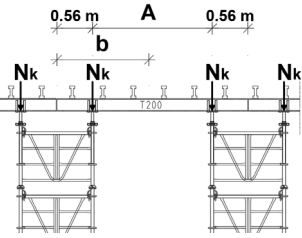
 <p><b>H20</b></p>	$M_{perm}: 5 \text{ kNm}$	$q_{SL}: g_F + g_{FC} + g_{CH} + v$	$q_{SL}: \text{Surface load}$
	$V_{perm}: 11 \text{ kN}$	$g_F: 0.25 \text{ kN/m}^2$	$g_F: \text{Dead weight formwork}$
	$EI: 500 \text{ kNm}^2$	$g_{FC}: 25 \text{ kN/m}^3 \times d/100$	$g_{FC}: \text{Dead weight fresh concrete}$
	$f_{perm}: L/500$	$g_{CH}: 0.10 * g_{FC}$ $0.75 \leq g_{CH} \leq 1.75 \text{ kN/m}^2$	$g_{CH}: \text{Concrete heaping}$
		$v: 0.75 \text{ kN/m}^2$	$v: \text{live load}$
 <p><b>DU-AL T200</b></p>	$M_{perm}: 20 \text{ kNm}$	$q_{SL}: g_F + g_{FC} + g_{CH} + v$	$q_{SL}: \text{Surface load}$
	$V_{perm}: 60 \text{ kN}$	$g_F: 0.25 \text{ kN/m}^2$	$g_F: \text{Dead weight formwork}$
	$EI: 954 \text{ kNm}^2$	$g_{FC}: 25 \text{ kN/m}^3 \times d/100$	$g_{FC}: \text{Dead weight fresh concrete}$
		$g_{CH}: 0.10 * g_{FC}$ $0.75 \leq g_{CH} \leq 1.75 \text{ kN/m}^2$	$g_{CH}: \text{Concrete heaping}$
	$f_{perm}: A/500$	$v: 0.75 \text{ kN/m}^2$	$v: \text{live load}$

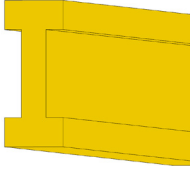
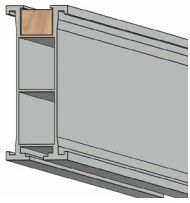
Slab thickness d	[cm]	20	25	30	35	40	45	50	55	60
Surface load $q_{SL}$	[kN/m <sup>2</sup> ]	<b>6.75</b>	<b>8.00</b>	<b>9.25</b>	<b>10.63</b>	<b>12.00</b>	<b>13.38</b>	<b>14.75</b>	<b>16.13</b>	<b>17.50</b>
 <p>Distance secondary beams e [m] 1.13 m L 1.13 m</p>	e [m]	<b>L permitted span of secondary beams [m] (timber beams H20)</b>								
	<b>0.33</b>	3.85	3.74	3.61	3.50	3.42	3.36	3.30	3.24	3.20
	<b>0.40</b>	3.71	3.58	3.48	3.39	3.31	3.24	3.19	3.14	3.10
	<b>0.50</b>	3.54	3.43	3.33	3.25	3.17	3.11	2.96	2.74	2.52
	<b>0.63</b>	3.39	3.28	3.20	3.12	2.92	2.60	2.34	2.17	2.00
	<b>0.67</b>	3.34	3.24	3.16	3.07	2.74	2.44	2.21	2.04	1.87
	<b>0.75</b>	3.27	3.19	3.11	2.77	2.44	2.19	1.97	1.82	1.67
	e [m]	<b>Resulting continuous load on primary beam [kN/m]</b>								
	<b>0.33</b>	20.60	23.94	27.27	30.61	33.94	37.57	41.21	44.24	47.87
	<b>0.40</b>	20.25	23.25	26.50	30.00	33.50	36.75	40.25	43.50	47.00
	<b>0.50</b>	19.60	22.80	25.80	29.20	32.60	36.00	38.40	38.60	38.80
	<b>0.63</b>	19.05	22.22	25.24	28.57	30.63	30.63	30.63	31.27	31.75
	<b>0.67</b>	18.95	21.94	25.07	28.21	28.81	28.81	29.10	29.70	30.15
	<b>0.75</b>	18.67	21.87	24.80	25.73	25.87	26.27	26.53	27.20	27.73
 <p>Selected span of secondary beams L [m] 0.56 m A 0.56 m</p>	L [m]	<b>A permitted span of primary beams [m] (single aluminium beam T200)</b>								
	<b>1.00</b>	2.84	2.71	2.60	2.50	2.41	2.35	2.29	2.24	2.19
	<b>1.25</b>	2.74	2.61	2.51	2.42	2.34	2.28	2.21	2.16	2.13
	<b>1.50</b>	2.66	2.54	2.43	2.35	2.28	2.20	2.15	2.11	2.06
	<b>1.75</b>	2.58	2.46	2.36	2.28	2.21	2.15	2.10	2.05	2.01
	<b>2.00</b>	2.50	2.39	2.30	2.23	2.15	2.09	2.04	2.00	1.96
	<b>2.25</b>	2.44	2.34	2.24	2.16	2.10	2.04	2.00	1.95	1.91
	<b>2.50</b>	2.38	2.28	2.19	2.10	2.05	2.00	1.95	1.91	1.88
 <p>Loading width [m] <math>b = A/2 + 0.56 \text{ m}</math> 0.56 m A 0.56 m</p>	L [m]	<b><math>N_k</math> resulting load per prop [kN]</b>								
	<b>1.00</b>	15.60	17.80	20.10	22.40	24.70	27.10	29.50	31.60	33.80
	<b>1.25</b>	17.20	19.70	22.10	24.60	27.30	29.90	32.50	34.80	37.40
	<b>1.50</b>	18.70	21.60	24.30	27.10	30.10	32.80	35.50	38.30	41.00
	<b>1.75</b>	20.50	23.40	26.60	29.60	32.80	36.00	39.10	41.80	44.90
	<b>2.00</b>	22.30	25.40	28.90	32.30	35.60	39.20	42.50	45.70	49.00
	<b>2.25</b>	24.00	27.70	31.20	34.90	38.80	42.50	46.30	49.60	53.20
	<b>2.50</b>	26.10	29.90	33.90	37.50	41.80	46.00	50.00	53.80	57.80

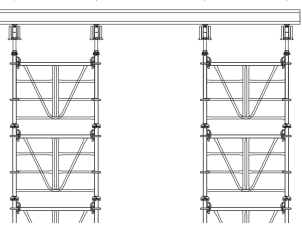
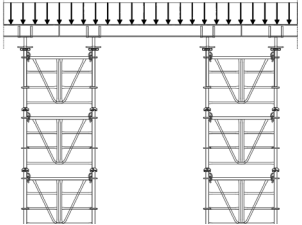
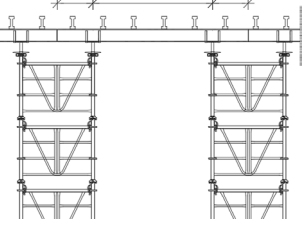
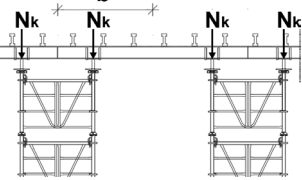
 <p><b>H20</b></p>	$M_{perm}: 5 \text{ kNm}$	$q_{SL}: g_F + g_{FC} + g_{CH} + v$	$q_{SL}: \text{Surface load}$
	$V_{perm}: 11 \text{ kN}$	$g_F: 0.25 \text{ kN/m}^2$	$g_F: \text{Dead weight formwork}$
	$EI: 500 \text{ kNm}^2$	$g_{FC}: 25 \text{ kN/m}^3 \times d/100$	$g_{FC}: \text{Dead weight fresh concrete}$
	$f_{perm}: L/500$	$g_{CH}: 0.10 * g_{FC}$ $0.75 \leq g_{CH} \leq 1.75 \text{ kN/m}^2$	$g_{CH}: \text{Concrete heaping}$
	$f_{perm}: A/500$	$v: 0.75 \text{ kN/m}^2$	$v: \text{live load}$

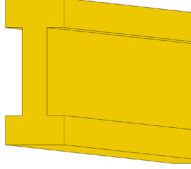
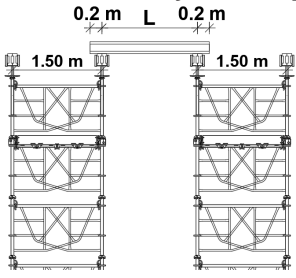
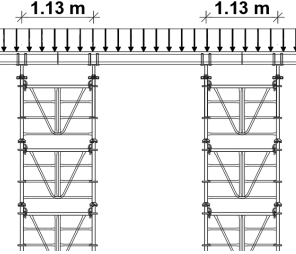
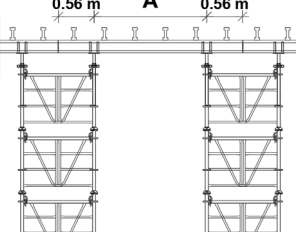
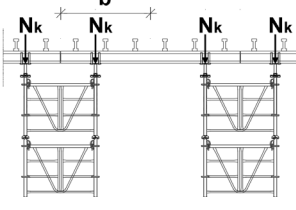
Slab thickness d	[cm]	20	25	30	35	40	45	50	55	60	
Surface load $q_{SL}$	[kN/m <sup>2</sup> ]	<b>6.75</b>	<b>8.00</b>	<b>9.25</b>	<b>10.63</b>	<b>12.00</b>	<b>13.38</b>	<b>14.75</b>	<b>16.13</b>	<b>17.50</b>	
<b>Distance secondary beams e [m]</b> 	e [m]	<b>L permitted span of secondary beams [m] (timber beams H20)</b>									
	<b>0.33</b>	3.85	3.74	3.61	3.50	3.42	3.36	3.30	3.24	3.20	
	<b>0.40</b>	3.71	3.58	3.48	3.39	3.31	3.24	3.19	3.14	3.10	
	<b>0.50</b>	3.54	3.43	3.33	3.25	3.17	3.11	2.96	2.74	2.52	
	<b>0.63</b>	3.39	3.28	3.20	3.12	2.92	2.60	2.34	2.17	2.00	
	<b>0.67</b>	3.34	3.24	3.16	3.07	2.74	2.44	2.21	2.04	1.87	
	<b>0.75</b>	3.27	3.19	3.11	2.77	2.44	2.19	1.97	1.82	1.67	
	e [m]	<b>Resulting continuous load on primary beam [kN/m]</b>									
	<b>0.33</b>	20.60	23.94	27.27	30.61	33.94	37.57	41.21	44.24	47.87	
	<b>0.40</b>	20.25	23.25	26.50	30.00	33.50	36.75	40.25	43.50	47.00	
	<b>0.50</b>	19.60	22.80	25.80	29.20	32.60	36.00	38.40	38.60	38.80	
	<b>0.63</b>	19.05	22.22	25.24	28.57	30.63	30.63	30.63	31.27	31.75	
	<b>0.67</b>	18.95	21.94	25.07	28.21	28.81	28.81	29.10	29.70	30.15	
	<b>0.75</b>	18.67	21.87	24.80	25.73	25.87	26.27	26.53	27.20	27.73	
<b>Selected span of secondary beams L [m]</b> 	L [m]	<b>A permitted span of primary beams [m] (double timber beams H20)</b>									
	<b>1.00</b>	2.88	2.75	2.63	2.53	2.45	2.38	2.31	2.25	2.15	
	<b>1.25</b>	2.78	2.65	2.54	2.45	2.38	2.30	2.24	2.08	1.91	
	<b>1.50</b>	2.70	2.56	2.46	2.38	2.30	2.21	2.03	1.86	1.70	
	<b>1.75</b>	2.61	2.49	2.39	2.31	2.23	1.99	1.81	1.67	1.53	
	<b>2.00</b>	2.54	2.43	2.33	2.25	2.02	1.80	1.63	1.50	1.38	
	<b>2.25</b>	2.48	2.36	2.28	2.07	1.82	1.63	1.48	1.36	1.24	
	<b>2.50</b>	2.41	2.30	2.15	1.89	1.66	1.49	1.35	1.24	1.14	
<b>Loading width [m]</b> $b = A/2 + 0.56 \text{ m}$ 	L [m]	<b><math>N_k</math> resulting load per prop [kN]</b>									
	<b>1.00</b>	15.80	18.00	20.30	22.60	25.00	27.30	29.70	31.70	33.40	
	<b>1.25</b>	17.40	19.90	22.30	24.80	27.70	30.10	32.80	33.90	34.80	
	<b>1.50</b>	18.90	21.70	24.50	27.30	30.30	32.90	34.20	35.30	36.40	
	<b>1.75</b>	20.70	23.60	26.90	29.80	33.00	34.20	35.60	36.80	38.00	
	<b>2.00</b>	22.50	25.80	29.20	32.50	34.20	35.60	37.00	38.40	39.80	
	<b>2.25</b>	24.30	27.80	31.60	34.00	35.40	37.00	38.60	40.10	41.40	
	<b>2.50</b>	26.30	30.10	33.50	35.10	36.70	38.50	40.30	41.90	43.50	

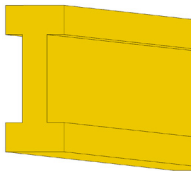
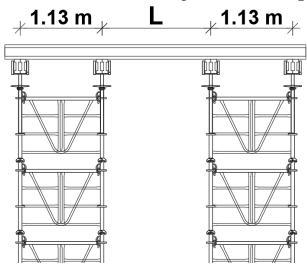
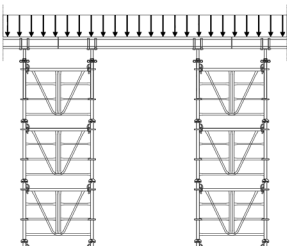
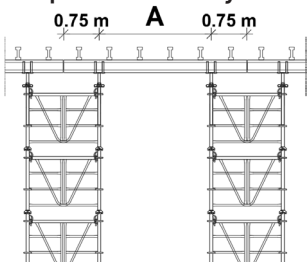
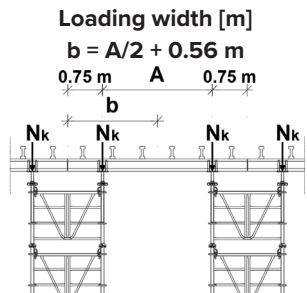
 <p><b>H20</b></p>	$M_{perm}: 5 \text{ kNm}$	$q_{SL}: g_F + g_{FC} + g_{CH} + v$	$q_{SL}: \text{Surface load}$
	$V_{perm}: 11 \text{ kN}$	$g_F: 0.25 \text{ kN/m}^2$	$g_F: \text{Dead weight formwork}$
	$EI: 500 \text{ kNm}^2$	$g_{FC}: 25 \text{ kN/m}^3 \times d/100$	$g_{FC}: \text{Dead weight fresh concrete}$
	$f_{perm}: L/500$	$g_{CH}: 0.10 \cdot g_{FC}$ $0.75 \leq g_{CH} \leq 1.75 \text{ kN/m}^2$	$g_{CH}: \text{Concrete heaping}$
		$v: 0.75 \text{ kN/m}^2$	$v: \text{live load}$
 <p><b>DU-AL T200</b></p>	$M_{perm}: 20 \text{ kNm}$	$q_{SL}: g_F + g_{FC} + g_{CH} + v$	$q_{SL}: \text{Surface load}$
	$V_{perm}: 60 \text{ kN}$	$g_F: 0.25 \text{ kN/m}^2$	$g_F: \text{Dead weight formwork}$
	$EI: 954 \text{ kNm}^2$	$g_{FC}: 25 \text{ kN/m}^3 \times d/100$	$g_{FC}: \text{Dead weight fresh concrete}$
		$g_{CH}: 0.10 \cdot g_{FC}$ $0.75 \leq g_{CH} \leq 1.75 \text{ kN/m}^2$	$g_{CH}: \text{Concrete heaping}$
	$f_{perm}: A/500$	$v: 0.75 \text{ kN/m}^2$	$v: \text{live load}$

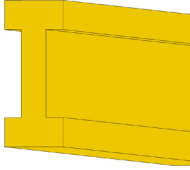
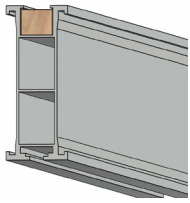
Slab thickness d	[cm]	20	25	30	35	40	45	50	55	60	
Surface load $q_{SL}$	[kN/m <sup>2</sup> ]	<b>6.75</b>	<b>8.00</b>	<b>9.25</b>	<b>10.63</b>	<b>12.00</b>	<b>13.38</b>	<b>14.75</b>	<b>16.13</b>	<b>17.50</b>	
Distance secondary beams e [m] $0.2 \text{ m} \quad L \quad 0.2 \text{ m}$ 	e [m]	<b>L permitted span of secondary beams [m] (timber beams H20)</b>									
	<b>0.33</b>	3.25	3.10	2.95	2.82	2.71	2.61	2.54	2.46	2.40	
	<b>0.40</b>	3.08	2.90	2.78	2.65	2.55	2.46	2.39	2.31	2.25	
	<b>0.50</b>	2.86	2.70	2.58	2.46	2.38	2.29	2.21	2.15	2.10	
	<b>0.63</b>	2.65	2.50	2.39	2.29	2.20	2.13	2.05	2.00	1.95	
	<b>0.67</b>	2.60	2.45	2.34	2.24	2.15	2.08	2.01	1.95	1.88	
	<b>0.75</b>	2.50	2.38	2.25	2.16	2.08	2.00	1.94	1.83	1.68	
	e [m]	<b>Resulting continuous load on primary beam [kN/m]</b>									
	<b>0.33</b>	16.03	18.40	20.58	22.96	25.26	27.50	29.80	31.94	34.13	
	<b>0.40</b>	15.46	17.60	19.80	22.06	24.30	26.49	28.69	30.73	32.81	
	<b>0.50</b>	14.72	16.80	18.87	21.05	23.28	25.36	27.36	29.44	31.50	
	<b>0.63</b>	14.01	16.00	17.99	20.14	22.20	24.28	26.18	28.23	30.19	
	<b>0.67</b>	13.84	15.80	17.76	19.88	21.90	23.95	25.89	27.82	29.58	
	<b>0.75</b>	13.50	15.52	17.34	19.45	21.48	23.42	25.37	26.86	27.83	
Selected span of secondary beams L [m] $0.56 \text{ m} \quad A \quad 0.56 \text{ m}$ 	L [m]	<b>A permitted span of primary beams [m] (single aluminium beam T200)</b>									
	<b>1.00</b>	2.79	2.65	2.55	2.45	2.38	2.30	2.25	2.20	2.15	
	<b>1.25</b>	2.71	2.59	2.49	2.39	2.31	2.25	2.19	2.14	2.10	
	<b>1.50</b>	2.64	2.53	2.43	2.34	2.26	2.20	2.14	2.10	2.05	
	<b>1.75</b>	2.59	2.46	2.38	2.29	2.21	2.15	2.10	2.05	2.01	
	<b>2.00</b>	2.54	2.41	2.33	2.24	2.18	2.11	2.06	2.02	1.98	
	<b>2.25</b>	2.49	2.38	2.29	2.20	2.14	2.08	2.03	1.99	1.95	
	<b>2.50</b>	2.44	2.34	2.25	2.16	2.10	2.05	2.00	1.96	1.92	
Loading width [m] $b = A/2 + 0.56 \text{ m}$ $0.56 \text{ m} \quad A \quad 0.56 \text{ m}$ 	L [m]	<b><math>N_k</math> resulting load per prop [kN]</b>									
	<b>1.00</b>	16.50	18.90	21.20	23.70	26.30	28.60	31.10	33.50	35.80	
	<b>1.25</b>	17.80	20.30	22.90	25.70	28.30	31.00	33.60	36.20	38.70	
	<b>1.50</b>	19.00	21.90	24.60	27.70	30.40	33.30	36.10	39.00	41.60	
	<b>1.75</b>	20.30	23.30	26.30	29.40	32.50	35.50	38.60	41.50	44.50	
	<b>2.00</b>	21.60	24.70	27.90	31.20	34.70	37.80	41.00	44.30	47.50	
	<b>2.25</b>	22.90	26.30	29.60	33.10	36.70	40.10	43.60	47.00	50.40	
<b>2.50</b>	24.00	27.70	31.20	34.90	38.60	42.40	46.00	49.70	53.20		

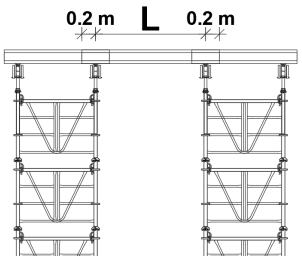
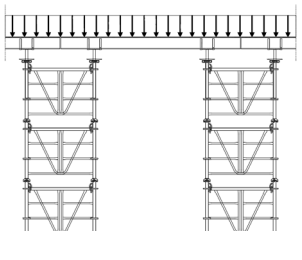
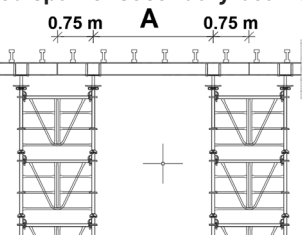
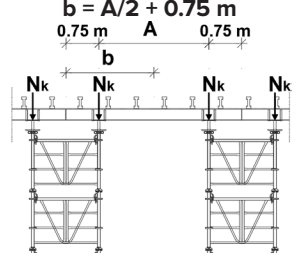
 <p><b>H20</b></p>	$M_{perm}: 5 \text{ kNm}$	$q_{SL}: g_F + g_{FC} + g_{CH} + v$	$q_{SL}: \text{Surface load}$
	$V_{perm}: 11 \text{ kN}$	$g_F: 0.25 \text{ kN/m}^2$	$g_F: \text{Dead weight formwork}$
	$EI: 500 \text{ kNm}^2$	$g_{FC}: 25 \text{ kN/m}^3 \times d/100$	$g_{FC}: \text{Dead weight fresh concrete}$
	$f_{perm}: L/500$	$g_{CH}: 0.10 \cdot g_{FC}$ $0.75 \leq g_{CH} \leq 1.75 \text{ kN/m}^2$	$g_{CH}: \text{Concrete heaping}$
		$v: 0.75 \text{ kN/m}^2$	$v: \text{live load}$
 <p><b>DU-AL T200</b></p>	$M_{perm}: 20 \text{ kNm}$	$q_{SL}: g_F + g_{FC} + g_{CH} + v$	$q_{SL}: \text{Surface load}$
	$V_{perm}: 60 \text{ kN}$	$g_F: 0.25 \text{ kN/m}^2$	$g_F: \text{Dead weight formwork}$
	$EI: 954 \text{ kNm}^2$	$g_{FC}: 25 \text{ kN/m}^3 \times d/100$	$g_{FC}: \text{Dead weight fresh concrete}$
		$g_{CH}: 0.10 \cdot g_{FC}$ $0.75 \leq g_{CH} \leq 1.75 \text{ kN/m}^2$	$g_{CH}: \text{Concrete heaping}$
	$f_{perm}: A/500$	$v: 0.75 \text{ kN/m}^2$	$v: \text{live load}$

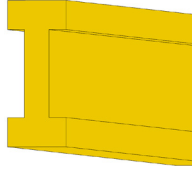
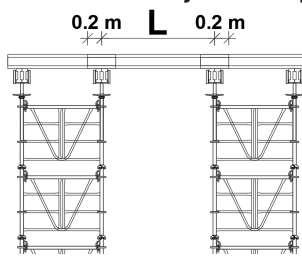
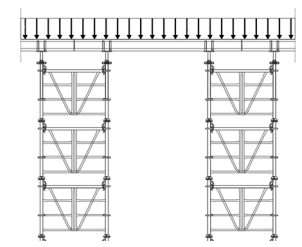
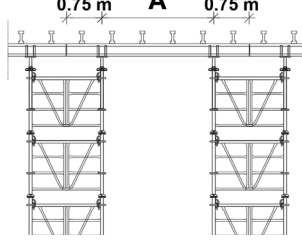
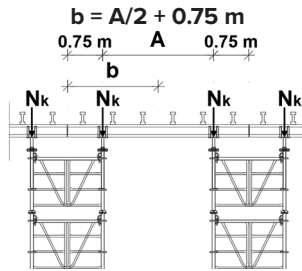
Slab thickness d	[cm]	20	25	30	35	40	45	50	55	60	
Surface load $q_{SL}$	[kN/m <sup>2</sup> ]	<b>6.75</b>	<b>8.00</b>	<b>9.25</b>	<b>10.63</b>	<b>12.00</b>	<b>13.38</b>	<b>14.75</b>	<b>16.13</b>	<b>17.50</b>	
Distance secondary beams e [m] $1.13 \text{ m} \quad L \quad 1.13 \text{ m}$ 	e [m]	<b>L permitted span of secondary beams [m] (timber beams H20)</b>									
	<b>0.33</b>	3.85	3.74	3.61	3.50	3.42	3.36	3.30	3.24	3.20	
	<b>0.40</b>	3.71	3.58	3.48	3.39	3.31	3.24	3.19	3.14	3.10	
	<b>0.50</b>	3.54	3.43	3.33	3.25	3.17	3.11	2.96	2.74	2.52	
	<b>0.63</b>	3.39	3.28	3.20	3.12	2.92	2.60	2.34	2.17	2.00	
	<b>0.67</b>	3.34	3.24	3.16	3.07	2.74	2.44	2.21	2.04	1.87	
	<b>0.75</b>	3.27	3.19	3.11	2.77	2.44	2.19	1.97	1.82	1.67	
	e [m]	<b>Resulting continuous load on primary beam [kN/m]</b>									
	<b>0.33</b>	20.60	23.94	27.27	30.61	33.94	37.57	41.21	44.24	47.87	
	<b>0.40</b>	20.25	23.25	26.50	30.00	33.50	36.75	40.25	43.50	47.00	
	<b>0.50</b>	19.60	22.80	25.80	29.20	32.60	36.00	38.40	38.60	38.80	
	<b>0.63</b>	19.05	22.22	25.24	28.57	30.63	30.63	30.63	31.27	31.75	
	<b>0.67</b>	18.95	21.94	25.07	28.21	28.81	28.81	29.10	29.70	30.15	
	<b>0.75</b>	18.67	21.87	24.80	25.73	25.87	26.27	26.53	27.20	27.73	
Selected span of secondary beams L [m] $0.75 \text{ m} \quad A \quad 0.75 \text{ m}$ 	L [m]	<b>A permitted span of primary beams [m] (single aluminium beam T200)</b>									
	<b>1.00</b>	2.99	2.86	2.76	2.68	2.60	2.53	2.47	2.43	2.39	
	<b>1.25</b>	2.90	2.78	2.68	2.60	2.53	2.46	2.41	2.37	2.33	
	<b>1.50</b>	2.82	2.70	2.61	2.53	2.46	2.40	2.36	2.31	2.28	
	<b>1.75</b>	2.74	2.64	2.54	2.47	2.40	2.35	2.30	2.26	2.23	
	<b>2.00</b>	2.68	2.58	2.49	2.41	2.35	2.30	2.26	2.22	2.19	
	<b>2.25</b>	2.61	2.51	2.44	2.36	2.31	2.26	2.21	2.19	2.05	
	<b>2.50</b>	2.56	2.46	2.39	2.32	2.26	2.22	2.17	—	—	
Loading width [m] $b = A/2 + 0.75 \text{ m}$ $0.75 \text{ m} \quad A \quad 0.75 \text{ m}$ 	L [m]	<b><math>N_k</math> resulting load per prop [kN]</b>									
	<b>1.00</b>	17.70	20.30	23.00	25.90	28.70	31.40	34.30	36.90	39.70	
	<b>1.25</b>	19.60	22.50	25.50	28.50	31.80	34.80	38.10	41.00	44.00	
	<b>1.50</b>	21.40	24.80	28.20	31.40	35.00	38.40	41.90	45.10	48.80	
	<b>1.75</b>	23.50	27.10	30.90	34.50	38.40	42.40	46.20	49.60	53.50	
	<b>2.00</b>	25.70	29.60	33.70	37.70	42.00	46.40	50.60	54.50	58.70	
	<b>2.25</b>	27.70	32.10	36.60	41.10	45.90	50.60	55.10	59.60	62.30	
	<b>2.50</b>	30.20	34.80	39.90	44.50	49.60	54.90	59.80	—	—	

 <p><b>H20</b></p>		$M_{perm}: 5 \text{ kNm}$		$q_{SL}: g_F + g_{FC} + g_{CH} + v$				$q_{SL}: \text{Surface load}$			
		$V_{perm}: 11 \text{ kN}$		$g_F: 0.25 \text{ kN/m}^2$				$g_F: \text{Dead weight formwork}$			
		$EI: 500 \text{ kNm}^2$		$g_{FC}: 25 \text{ kN/m}^3 \times d/100$				$g_{FC}: \text{Dead weight fresh concrete}$			
		$f_{perm}: L/500$		$g_{CH}: 0.10 * g_{FC}$ $0.75 \leq g_{CH} \leq 1.75 \text{ kN/m}^2$				$g_{CH}: \text{Concrete heaping}$			
		$f_{perm}: A/500$		$v: 0.75 \text{ kN/m}^2$				$v: \text{live load}$			
<b>Slab thickness d</b>	[cm]	20	25	30	35	40	45	50	55	60	
<b>Surface load <math>q_{SL}</math></b>	[kN/m <sup>2</sup> ]	<b>6.75</b>	<b>8.00</b>	<b>9.25</b>	<b>10.63</b>	<b>12.00</b>	<b>13.38</b>	<b>14.75</b>	<b>16.13</b>	<b>17.50</b>	
<b>Distance secondary beams e [m]</b> 	<b>e [m]</b>	<b>L permitted span of secondary beams [m] (timber beams H20)</b>									
	<b>0.33</b>	3.25	3.10	2.95	2.82	2.71	2.61	2.54	2.46	2.40	
	<b>0.40</b>	3.08	2.90	2.78	2.65	2.55	2.46	2.39	2.31	2.25	
	<b>0.50</b>	2.86	2.70	2.58	2.46	2.38	2.29	2.21	2.15	2.10	
	<b>0.63</b>	2.65	2.50	2.39	2.29	2.20	2.13	2.05	2.00	1.95	
	<b>0.67</b>	2.60	2.45	2.34	2.24	2.15	2.08	2.01	1.95	1.88	
	<b>0.75</b>	2.50	2.38	2.25	2.16	2.08	2.00	1.94	1.83	1.68	
	<b>e [m]</b>	<b>Resulting continuous load on primary beam [kN/m]</b>									
	<b>0.33</b>	16.03	18.40	20.58	22.96	25.26	27.50	29.80	31.94	34.13	
	<b>0.40</b>	15.46	17.60	19.80	22.06	24.30	26.49	28.69	30.73	32.81	
	<b>0.50</b>	14.72	16.80	18.87	21.05	23.28	25.36	27.36	29.44	31.50	
	<b>0.63</b>	14.01	16.00	17.99	20.14	22.20	24.28	26.18	28.23	30.19	
	<b>0.67</b>	13.84	15.80	17.76	19.88	21.90	23.95	25.89	27.82	29.58	
	<b>0.75</b>	13.50	15.52	17.34	19.45	21.48	23.42	25.37	26.86	27.83	
<b>Selected span of secondary beams L [m]</b> 	<b>L [m]</b>	<b>A permitted span of primary beams [m] (double timber beams H20)</b>									
	<b>1.00</b>	2.83	2.69	2.57	2.47	2.40	2.34	2.28	2.18	2.01	
	<b>1.25</b>	2.75	2.61	2.51	2.41	2.34	2.28	2.17	1.98	1.83	
	<b>1.50</b>	2.68	2.55	2.45	2.36	2.29	2.19	1.99	1.82	1.67	
	<b>1.75</b>	2.61	2.50	2.40	2.31	2.24	2.03	1.83	1.68	1.54	
	<b>2.00</b>	2.56	2.45	2.35	2.28	2.09	1.88	1.70	1.56	1.43	
	<b>2.25</b>	2.51	2.40	2.31	2.21	1.95	1.75	1.59	1.45	1.34	
	<b>2.50</b>	2.48	2.36	2.28	2.07	1.83	1.64	1.49	1.36	1.25	
<b>Loading width [m]</b> $b = A/2 + 0.56 \text{ m}$ 	<b>L [m]</b>	<b><math>N_k</math> resulting load per prop [kN]</b>									
	<b>1.00</b>	16.70	19.10	21.30	23.90	26.40	28.90	31.50	33.30	34.30	
	<b>1.25</b>	18.00	20.50	23.10	25.80	28.50	31.30	33.40	34.40	35.50	
	<b>1.50</b>	19.20	22.00	24.80	27.80	30.70	33.30	34.40	35.60	36.60	
	<b>1.75</b>	20.50	23.50	26.40	29.70	32.80	34.20	35.40	36.70	37.80	
	<b>2.00</b>	21.70	25.00	28.10	31.60	33.70	35.10	36.40	37.80	39.10	
	<b>2.25</b>	23.10	26.40	29.70	33.20	34.50	36.00	37.50	38.90	40.40	
	<b>2.50</b>	24.30	27.80	31.50	34.00	35.40	36.90	38.50	40.00	41.50	

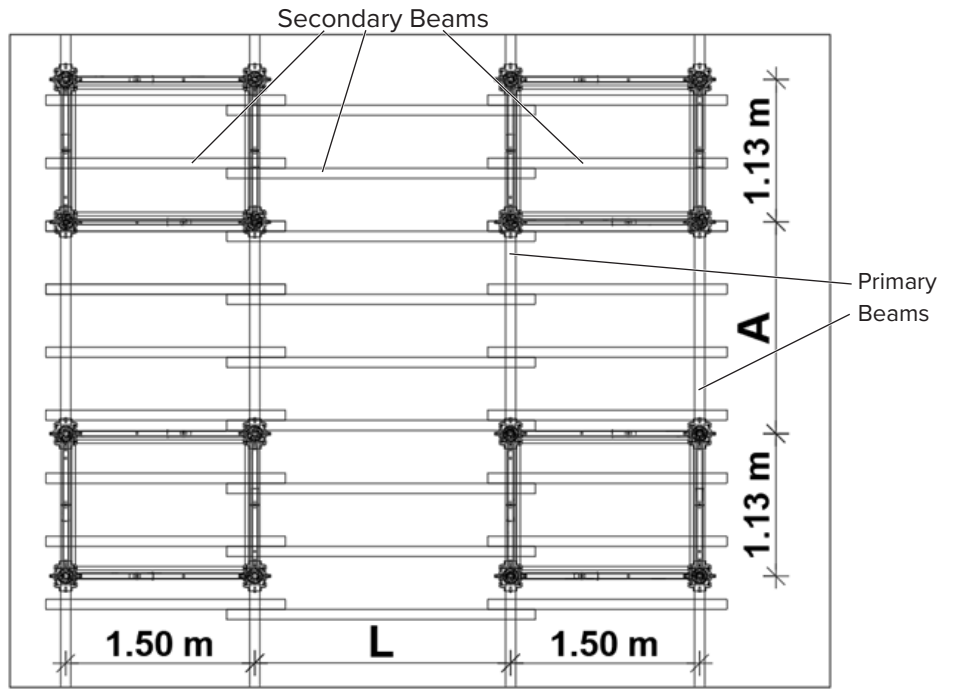
 <p><b>H20</b></p>	$M_{perm}$ : 5 kNm	$q_{SL}$ : $g_F + g_{FC} + g_{CH} + v$	$q_{SL}$ : Surface load							
	$V_{perm}$ : 11 kN	$g_F$ : 0.25 kN/m <sup>2</sup>	$g_F$ : Dead weight formwork							
	$EI$ : 500 kNm <sup>2</sup>	$g_{FC}$ : 25 kN/m <sup>3</sup> x d/100	$g_{FC}$ : Dead weight fresh concrete							
	$f_{perm}$ : L/500	$g_{CH}$ : 0.10 * $g_{FC}$ $0.75 \leq g_{CH} \leq 1.75$ kN/m <sup>2</sup>	$g_{CH}$ : Concrete heaping							
	$f_{perm}$ : A/500	$v$ : 0.75 kN/m <sup>2</sup>	$v$ : live load							
<b>Slab thickness d</b>	[cm]	20	25	30	35	40	45	50	55	60
<b>Surface load <math>q_{SL}</math></b>	[kN/m <sup>2</sup> ]	<b>6.75</b>	<b>8.00</b>	<b>9.25</b>	<b>10.63</b>	<b>12.00</b>	<b>13.38</b>	<b>14.75</b>	<b>16.13</b>	<b>17.50</b>
<b>Distance secondary beams e [m]</b> 	<b>e [m]</b>	<b>L permitted span of secondary beams [m] (timber beams H20)</b>								
	<b>0.33</b>	3.85	3.74	3.61	3.50	3.42	3.36	3.30	3.24	3.20
	<b>0.40</b>	3.71	3.58	3.48	3.39	3.31	3.24	3.19	3.14	3.10
	<b>0.50</b>	3.54	3.43	3.33	3.25	3.17	3.11	2.96	2.74	2.52
	<b>0.63</b>	3.39	3.28	3.20	3.12	2.92	2.60	2.34	2.17	2.00
	<b>0.67</b>	3.34	3.24	3.16	3.07	2.74	2.44	2.21	2.04	1.87
	<b>0.75</b>	3.27	3.19	3.11	2.77	2.44	2.19	1.97	1.82	1.67
	<b>e [m]</b>	<b>Resulting continuous load on primary beam [kN/m]</b>								
	<b>0.33</b>	20.60	23.94	27.27	30.61	33.94	37.57	41.21	44.24	47.87
	<b>0.40</b>	20.25	23.25	26.50	30.00	33.50	36.75	40.25	43.50	47.00
	<b>0.50</b>	19.60	22.80	25.80	29.20	32.60	36.00	38.40	38.60	38.80
	<b>0.63</b>	19.05	22.22	25.24	28.57	30.63	30.63	30.63	31.27	31.75
	<b>0.67</b>	18.95	21.94	25.07	28.21	28.81	28.81	29.10	29.70	30.15
	<b>0.75</b>	18.67	21.87	24.80	25.73	25.87	26.27	26.53	27.20	27.73
<b>Selected span of secondary beams L [m]</b> 	<b>L [m]</b>	<b>A permitted span of primary beams [m] (double timber beams H20)</b>								
	<b>1.00</b>	3.03	2.90	2.79	2.70	2.63	2.56	2.50	2.34	2.16
	<b>1.25</b>	2.92	2.81	2.71	2.63	2.55	2.49	2.26	2.07	1.91
	<b>1.50</b>	2.85	2.74	2.64	2.56	2.49	2.23	2.03	1.85	1.70
	<b>1.75</b>	2.78	2.66	2.57	2.50	2.23	2.00	1.81	1.67	1.53
	<b>2.00</b>	2.70	2.60	2.51	2.28	2.02	1.80	1.63	1.50	—
	<b>2.25</b>	2.65	2.54	2.37	2.07	1.82	1.63	—	—	—
	<b>2.50</b>	2.59	2.49	2.15	1.89	1.67	—	—	—	—
<b>Loading width [m]</b> $b = A/2 + 0.56$ m 	<b>L [m]</b>	<b><math>N_k</math> resulting load per prop [kN]</b>								
	<b>1.00</b>	17.90	20.50	23.20	26.00	28.90	31.70	34.60	36.10	37.30
	<b>1.25</b>	19.70	22.60	25.70	28.70	32.00	35.10	36.70	37.80	39.20
	<b>1.50</b>	21.50	25.00	28.40	31.70	35.30	36.70	38.30	39.70	41.30
	<b>1.75</b>	23.80	27.20	31.10	34.80	36.70	38.50	40.20	41.80	43.50
	<b>2.00</b>	25.80	29.70	33.90	36.50	38.40	40.30	42.10	44.00	—
	<b>2.25</b>	28.00	32.30	36.00	38.00	40.00	42.10	—	—	—
	<b>2.50</b>	30.50	35.10	37.40	39.50	41.80	—	—	—	—

 <p><b>H20</b></p>	$M_{perm}: 5 \text{ kNm}$	$q_{SL}: g_F + g_{FC} + g_{CH} + v$	$q_{SL}: \text{Surface load}$
	$V_{perm}: 11 \text{ kN}$	$g_F: 0.25 \text{ kN/m}^2$	$g_F: \text{Dead weight formwork}$
	$EI: 500 \text{ kNm}^2$	$g_{FC}: 25 \text{ kN/m}^3 \times d/100$	$g_{FC}: \text{Dead weight fresh concrete}$
	$f_{perm}: L/500$	$g_{CH}: 0.10 * g_{FC}$ $0.75 \leq g_{CH} \leq 1.75 \text{ kN/m}^2$	$g_{CH}: \text{Concrete heaping}$
		$v: 0.75 \text{ kN/m}^2$	$v: \text{live load}$
 <p><b>DU-AL T200</b></p>	$M_{perm}: 20 \text{ kNm}$	$q_{SL}: g_F + g_{FC} + g_{CH} + v$	$q_{SL}: \text{Surface load}$
	$V_{perm}: 60 \text{ kN}$	$g_F: 0.25 \text{ kN/m}^2$	$g_F: \text{Dead weight formwork}$
	$EI: 954 \text{ kNm}^2$	$g_{FC}: 25 \text{ kN/m}^3 \times d/100$	$g_{FC}: \text{Dead weight fresh concrete}$
		$g_{CH}: 0.10 * g_{FC}$ $0.75 \leq g_{CH} \leq 1.75 \text{ kN/m}^2$	$g_{CH}: \text{Concrete heaping}$
	$f_{perm}: A/500$	$v: 0.75 \text{ kN/m}^2$	$v: \text{live load}$

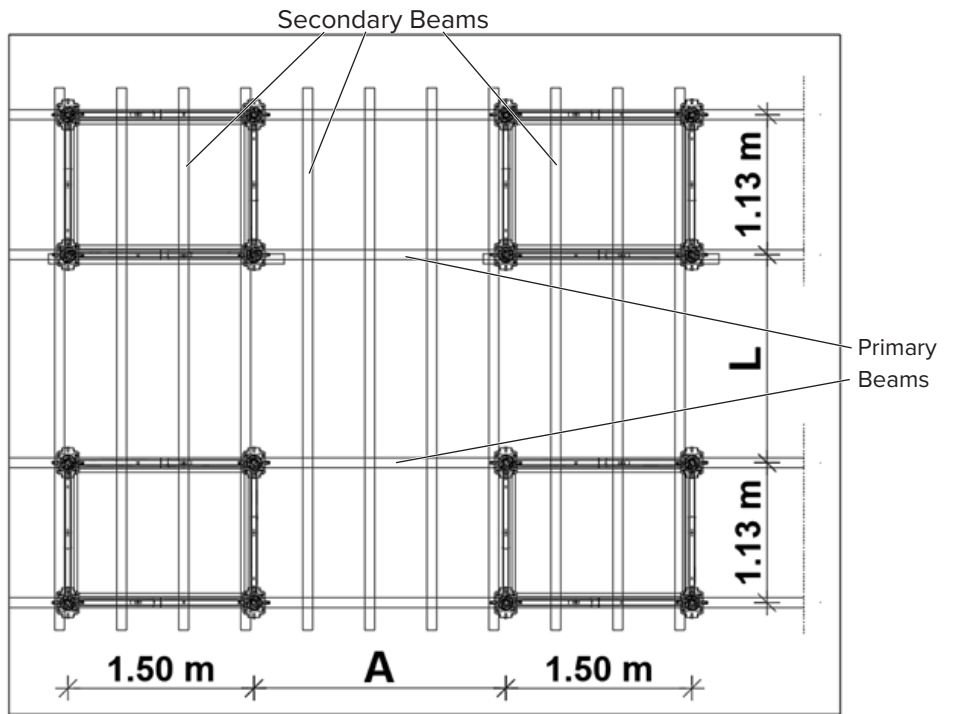
Slab thickness d	[cm]	20	25	30	35	40	45	50	55	60
Surface load $q_{SL}$	[kN/m <sup>2</sup> ]	<b>6.75</b>	<b>8.00</b>	<b>9.25</b>	<b>10.63</b>	<b>12.00</b>	<b>13.38</b>	<b>14.75</b>	<b>16.13</b>	<b>17.50</b>
Distance secondary beams e [m] 	e [m]	<b>L permitted span of secondary beams [m] (timber beams H20)</b>								
	<b>0.33</b>	3.25	3.10	2.95	2.82	2.71	2.61	2.54	2.46	2.40
	<b>0.40</b>	3.08	2.90	2.78	2.65	2.55	2.46	2.39	2.31	2.25
	<b>0.50</b>	2.86	2.70	2.58	2.46	2.38	2.29	2.21	2.15	2.10
	<b>0.63</b>	2.65	2.50	2.39	2.29	2.20	2.13	2.05	2.00	1.95
	<b>0.67</b>	2.60	2.45	2.34	2.24	2.15	2.08	2.01	1.95	1.88
	<b>0.75</b>	2.50	2.38	2.25	2.16	2.08	2.00	1.94	1.83	1.68
	e [m]	<b>Resulting continuous load on primary beam [kN/m]</b>								
	<b>0.33</b>	16.03	18.40	20.58	22.96	25.26	27.50	29.80	31.94	34.13
	<b>0.40</b>	15.46	17.60	19.80	22.06	24.30	26.49	28.69	30.73	32.81
	<b>0.50</b>	14.72	16.80	18.87	21.05	23.28	25.36	27.36	29.44	31.50
	<b>0.63</b>	14.01	16.00	17.99	20.14	22.20	24.28	26.18	28.23	30.19
	<b>0.67</b>	13.84	15.80	17.76	19.88	21.90	23.95	25.89	27.82	29.58
	<b>0.75</b>	13.50	15.52	17.34	19.45	21.48	23.42	25.37	26.86	27.83
Selected span of secondary beams L [m] 	L [m]	<b>A permitted span of primary beams [m] (single aluminium beam T200)</b>								
	<b>1.00</b>	2.93	2.81	2.71	2.62	2.55	2.49	2.44	2.39	2.35
	<b>1.25</b>	2.85	2.74	2.65	2.56	2.50	2.44	2.39	2.34	2.31
	<b>1.50</b>	2.80	2.68	2.60	2.51	2.45	2.39	2.34	2.30	2.27
	<b>1.75</b>	2.74	2.64	2.55	2.47	2.41	2.35	2.31	2.27	2.23
	<b>2.00</b>	2.70	2.59	2.51	2.43	2.37	2.32	2.27	2.23	2.20
	<b>2.25</b>	2.65	2.55	2.47	2.40	2.34	2.29	2.24	2.21	2.05
	<b>2.50</b>	2.61	2.51	2.43	2.36	2.31	2.26	2.22	2.15	1.87
Loading width [m] $b = A/2 + 0.75 \text{ m}$ 	L [m]	<b><math>N_k</math> resulting load per prop [kN]</b>								
	<b>1.00</b>	18.70	21.60	24.30	27.40	30.40	33.40	36.30	39.20	42.10
	<b>1.25</b>	20.20	23.30	26.40	29.70	33.00	36.20	39.40	42.60	45.80
	<b>1.50</b>	21.80	25.10	28.50	32.00	35.60	39.00	42.50	46.00	49.50
	<b>1.75</b>	23.30	26.90	30.40	34.30	38.10	41.80	45.70	49.40	53.00
	<b>2.00</b>	24.80	28.60	32.50	36.50	40.60	44.70	48.70	52.60	56.70
	<b>2.25</b>	26.40	30.40	34.40	38.90	43.20	47.50	51.70	56.10	58.20
<b>2.50</b>	27.70	32.10	36.40	41.00	45.70	50.30	54.90	58.90	59.00	

 <p><b>H20</b></p>	$M_{perm}$ : 5 kNm	$q_{SL}$ : $g_F + g_{FC} + g_{CH} + v$	$q_{SL}$ : Surface load							
	$V_{perm}$ : 11 kN	$g_F$ : 0.25 kN/m <sup>2</sup>	$g_F$ : Dead weight formwork							
	$EI$ : 500 kNm <sup>2</sup>	$g_{FC}$ : 25 kN/m <sup>3</sup> x d/100	$g_{FC}$ : Dead weight fresh concrete							
	$f_{perm}$ : L/500	$g_{CH}$ : 0.10 * $g_{FC}$ 0.75 ≤ $g_{CH}$ ≤ 1.75 kN/m <sup>2</sup>	$g_{CH}$ : Concrete heaping							
	$f_{perm}$ : A/500	$v$ : 0.75 kN/m <sup>2</sup>	$v$ : live load							
<b>Slab thickness d</b>	[cm]	20	25	30	35	40	45	50	55	60
<b>Surface load <math>q_{SL}</math></b>	[kN/m <sup>2</sup> ]	<b>6.75</b>	<b>8.00</b>	<b>9.25</b>	<b>10.63</b>	<b>12.00</b>	<b>13.38</b>	<b>14.75</b>	<b>16.13</b>	<b>17.50</b>
<b>Distance secondary beams e [m]</b> 	<b>e [m]</b>	<b>L permitted span of secondary beams [m] (timber beams H20)</b>								
	<b>0.33</b>	3.25	3.10	2.95	2.82	2.71	2.61	2.54	2.46	2.40
	<b>0.40</b>	3.08	2.90	2.78	2.65	2.55	2.46	2.39	2.31	2.25
	<b>0.50</b>	2.86	2.70	2.58	2.46	2.38	2.29	2.21	2.15	2.10
	<b>0.63</b>	2.65	2.50	2.39	2.29	2.20	2.13	2.05	2.00	1.95
	<b>0.67</b>	2.60	2.45	2.34	2.24	2.15	2.08	2.01	1.95	1.88
	<b>0.75</b>	2.50	2.38	2.25	2.16	2.08	2.00	1.94	1.83	1.68
	<b>e [m]</b>	<b>Resulting continuous load on primary beam [kN/m]</b>								
	<b>0.33</b>	16.03	18.40	20.58	22.96	25.26	27.50	29.80	31.94	34.13
	<b>0.40</b>	15.46	17.60	19.80	22.06	24.30	26.49	28.69	30.73	32.81
	<b>0.50</b>	14.72	16.80	18.87	21.05	23.28	25.36	27.36	29.44	31.50
	<b>0.63</b>	14.01	16.00	17.99	20.14	22.20	24.28	26.18	28.23	30.19
	<b>0.67</b>	13.84	15.80	17.76	19.88	21.90	23.95	25.89	27.82	29.58
	<b>0.75</b>	13.50	15.52	17.34	19.45	21.48	23.42	25.37	26.86	27.83
<b>Selected span of secondary beams L [m]</b> 	<b>L [m]</b>	<b>A permitted span of primary beams [m] (double timber beams H20)</b>								
	<b>1.00</b>	2.97	2.85	2.75	2.65	2.58	2.52	2.39	2.18	2.01
	<b>1.25</b>	2.90	2.78	2.69	2.60	2.53	2.39	2.17	1.98	1.83
	<b>1.50</b>	2.84	2.73	2.63	2.55	2.44	2.19	1.98	1.82	1.67
	<b>1.75</b>	2.78	2.67	2.58	2.50	2.26	2.02	1.83	1.68	1.55
	<b>2.00</b>	2.74	2.62	2.54	2.35	2.09	1.88	1.70	1.56	—
	<b>2.25</b>	2.69	2.58	2.50	2.20	1.95	1.75	1.59	—	—
	<b>2.50</b>	2.65	2.54	2.38	2.07	1.83	1.64	1.40	—	—
<b>Loading width [m]</b> $b = A/2 + 0.75$ m 	<b>L [m]</b>	<b><math>N_k</math> resulting load per prop [kN]</b>								
	<b>1.00</b>	18.90	21.80	24.60	27.60	30.60	33.60	35.90	37.10	38.40
	<b>1.25</b>	20.40	23.50	26.60	30.00	33.20	35.80	37.20	38.60	40.10
	<b>1.50</b>	22.00	25.40	28.70	32.30	35.50	37.00	38.50	40.20	41.60
	<b>1.75</b>	23.50	27.10	30.70	34.50	36.70	38.30	39.90	41.70	43.40
	<b>2.00</b>	25.00	28.80	32.70	35.80	37.70	39.60	41.30	43.20	—
	<b>2.25</b>	26.60	30.60	34.70	36.90	38.80	40.80	42.70	—	—
	<b>2.50</b>	28.00	32.30	35.90	37.90	40.00	42.00	43.10	—	—

Drawing 1



Drawing 2



**Example for the use of load table for ST60 1.13 m x 1.13 m**

In this example, the use of the load table for ST60 1.13 m x 1.13 m, secondary beams timber H20 and primary beams 1 x DU-AL T200 (refer to page 4) is described.

Assumption: Classification class B1 acc. to EN 12812:2008.

**Assumptions for shoring example**

Slab thickness (1):	d = 35 cm
Secondary beams:	H20 timber beams (3 spans, 4 supports, 2 cantilevers)
Secondary beam spacing (2):	e = 0.40 m
Primary beams:	DU-AL T200 aluminium beams (1 span, 2 supports, 2 cantilevers)
ST 60 towers:	1.13m x 1.13m

**Determining the max. permissible span for secondary beams**

In part “A” of the example table (refer to page 14) find where the 35 cm slab thickness column (1) intersects with the 0.40 m secondary spacing row (2) to obtain the maximum permitted span of secondary beams of 3.39 m (3) (= max. primary beam spacing).

In part “B” of the table, obtain the resulting continuous load on primary beam 30.00 kN/m (4).

**Determining the max. permissible span for primary beams**

In the next stage of dimensioning, we select the span of secondary beams “L”, which must not exceed the value 3.39 m (3).

The value “L” is assumed 2.50 m (5).

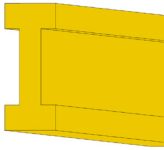
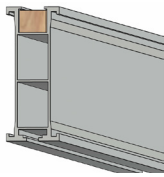
In part “C” of the table find where the 35 cm (1) slab thickness column intersects with the 2.50 m (5) row of selected span of secondary beams “L”, to obtain the maximum permitted span of primary beams A = 2.10 m (6) (= max. distance between ST60 props).

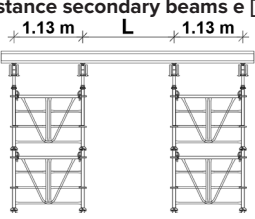
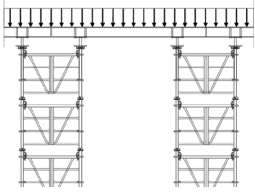
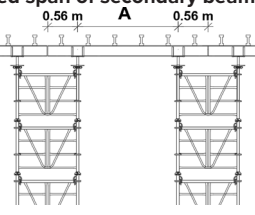
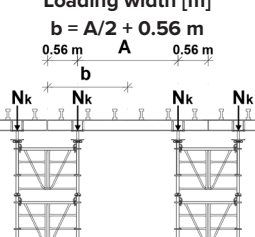
In part “D” of the table for L = 2.50 m (5) and A = 2.10 m (6) the resulting load per prop will be 37.50 kN (7).

**Checking the load capacity of ST60 props**

The resulting load per prop 37.50 kN (7) must be compared with the maximum permissible vertical load for the ST60 prop depicted in diagrams in the ST 60 user guide (depending on the working scheme, tower height, wind load, jacks extensions, etc.).

Example load table for ST60 1.13 m x 1.13 m  
Secondary beams timber H20 and primary beams 1 x DU-AL T200

 <p><b>H20</b></p>	$M_{perm}: 5 \text{ kNm}$	$q_{SL}: g_F + g_{FC} + g_{CH} + v$	$q_{SL}: \text{Surface load}$
	$V_{perm}: 11 \text{ kN}$	$g_F: 0.25 \text{ kN/m}^2$	$g_F: \text{Dead weight formwork}$
	$EI: 500 \text{ kNm}^2$	$g_{FC}: 25 \text{ kN/m}^3 \times d/100$	$g_{FC}: \text{Dead weight fresh concrete}$
	$f_{perm}: L/500$	$g_{CH}: 0.10 \cdot g_{FC}$ $0.75 \leq g_{CH} \leq 1.75 \text{ kN/m}^2$	$g_{CH}: \text{Concrete heaping}$
 <p><b>DU-AL T200</b></p>	$M_{perm}: 20 \text{ kNm}$	$q_{SL}: g_F + g_{FC} + g_{CH} + v$	$q_{SL}: \text{Surface load}$
	$V_{perm}: 60 \text{ kN}$	$g_F: 0.25 \text{ kN/m}^2$	$g_F: \text{Dead weight formwork}$
	$EI: 954 \text{ kNm}^2$	$g_{FC}: 25 \text{ kN/m}^3 \times d/100$	$g_{FC}: \text{Dead weight fresh concrete}$
		$g_{CH}: 0.10 \cdot g_{FC}$ $0.75 \leq g_{CH} \leq 1.75 \text{ kN/m}^2$	$g_{CH}: \text{Concrete heaping}$
	$f_{perm}: A/500$	$v: 0.75 \text{ kN/m}^2$	$v: \text{live load}$

	<b>Slab thickness d</b>	[cm]	20	25	30	35	40	45	50	55	60
	<b>Surface load <math>q_{SL}</math></b>	[kN/m <sup>2</sup> ]	6.75	8.00	9.25	10.63	12.00	13.38	14.75	16.13	17.50
<b>A</b> 	<b>Distance secondary beams e [m]</b>	<b>L permitted span of secondary beams [m] (timber beams H20)</b>									
	<b>0.33</b>	3.85	3.74	3.61	3.50	3.42	3.36	3.30	3.24	3.20	
	<b>0.40</b>	3.71	3.58	3.48	3.39	3.31	3.24	3.19	3.14	3.10	
	<b>0.50</b>	3.54	3.43	3.33	3.25	3.17	3.11	2.96	2.74	2.52	
	<b>0.63</b>	3.39	3.28	3.20	3.12	2.92	2.60	2.34	2.17	2.00	
	<b>0.67</b>	3.34	3.24	3.16	3.07	2.74	2.44	2.21	2.04	1.87	
	<b>0.75</b>	3.27	3.19	3.11	2.77	2.44	2.19	1.97	1.82	1.67	
<b>B</b> 	<b>Resulting continuous load on primary beam [kN/m]</b>										
	<b>0.33</b>	20.60	23.94	27.27	30.61	33.94	37.57	41.21	44.24	47.87	
	<b>0.40</b>	20.25	23.25	26.50	30.00	33.50	36.75	40.25	43.50	47.00	
	<b>0.50</b>	19.60	22.80	25.80	29.20	32.60	36.00	38.40	38.60	38.80	
	<b>0.63</b>	19.05	22.22	25.24	28.57	30.63	30.63	30.63	31.27	31.75	
	<b>0.67</b>	18.95	21.94	25.07	28.21	28.81	28.81	29.10	29.70	30.15	
<b>C</b> 	<b>Selected span of secondary beams L [m]</b>	<b>A permitted span of primary beams [m] (single aluminium beam T200)</b>									
	<b>1.00</b>	2.84	2.71	2.60	2.50	2.41	2.35	2.29	2.24	2.19	
	<b>1.25</b>	2.74	2.61	2.51	2.42	2.34	2.28	2.21	2.16	2.13	
	<b>1.50</b>	2.66	2.54	2.43	2.35	2.28	2.20	2.15	2.11	2.06	
	<b>1.75</b>	2.58	2.46	2.36	2.28	2.21	2.15	2.10	2.05	2.01	
	<b>2.00</b>	2.50	2.39	2.30	2.23	2.15	2.09	2.04	2.00	1.96	
	<b>2.25</b>	2.44	2.34	2.24	2.16	2.10	2.04	2.00	1.95	1.91	
	<b>2.50</b>	2.38	2.28	2.19	2.10	2.05	2.00	1.95	1.91	1.88	
<b>D</b> 	<b>Resulting load per prop [kN]</b>										
	<b>1.00</b>	15.60	17.80	20.10	22.40	24.70	27.10	29.50	31.60	33.80	
	<b>1.25</b>	17.20	19.70	22.10	24.60	27.30	29.90	32.50	34.80	37.40	
	<b>1.50</b>	18.70	21.60	24.30	27.10	30.10	32.80	35.50	38.30	41.00	
	<b>1.75</b>	20.50	23.40	26.60	29.60	32.80	36.00	39.10	41.80	44.90	
	<b>2.00</b>	22.30	25.40	28.90	32.30	35.60	39.20	42.50	45.70	49.00	
	<b>2.25</b>	24.00	27.70	31.20	34.90	38.80	42.50	46.30	49.60	53.20	
<b>2.50</b>	26.10	29.90	33.90	37.50	41.80	46.00	50.00	53.80	57.80		